An Experimental Study On Soil Stabilisation Using Composite Stabilizing Agents

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ABSTRACT

Present study deals with the stabilization of black cotton soil which is highly prone to settlement. It involves the stabilization of soil with admixtures, lime, quarry dust and fly ash. The concentration of lime is kept constant as 6% but the quarry dust and fly ash quantities are varied by 5% in every test sample. The maximum concentration of quarry dust is 25% and for fly ash it is 20%. It involves the study of behavior of the sample under varying concentration of admixtures and the increase in the strength of the sample. The classification of soil and its testing are done in accordance with the Indian Standard code IS 2720 -1985. The bearing capacity of the samples is determined by the California Bearing Ratio test. The sample showed the improved stability and bearing capacity with the addition of sustained amount of admixtures and the test results showed that lime, fly ash and quarry dust can be used together as an effective stabilizing agent to increase the load resisting capacity of the clay sample.

Keywords:

I. INTRODUCTION

Government of India is planning a new corridor electric trucks and proposals for new road projects connecting different parts of the country. The NHAI is bidding to develop two lane highways to multilane highways. So the future sees a vast investment in the road construction. These proposed roads lies in different soil conditions, some under severe clayey conditions such as those in the areas like Pattikad, Mannuthy where the development of NH47 is going on. So there is a necessary to stabilize these clayey conditions with very effective soil stabilization technique which is economical as well. Various methods can be employed to stabilize the soil like Dewatering, Compaction, providing Reinforcements etc.., but the most economical way is the usage stabilizing agents, especially in case of Roads. Clayey soil gets its expansive nature due to the presence of minerals like illite, montmorillonite, chlorite, hallosite, and smectite. The swelling and shrinkage nature of the Black Cotton Soil leads to various failures in pavements. And hence altering such properties would effectively yield a stratum which can be subjected to higher loads. Many attempts were made to stabilize the soil sample, lime being the most commonly used admixture. We have conducted experiments to study the behavior of BC soil with the usage of a composite mixture of admixtures. The percentage of lime is fixed at 6% as per previous studies, and the behavior is studied with varied percentages fro quarry dust and fly ash.

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II. MATERIALS

A. SOIL SAMPLE

The soil sample is collected from Kumaraguru College of Technology, Coimbatore, TamilNadu. The top soil stratum is dug up to 1m depth and the samples are collected. In order to classify the soil sample the Indian Standard Code IS24720-1985 is referred.

B. LIME

Powdered Lime is collected directly from kilns near Kumaraguru College of Technology. The composition of powdered lime is listed below in Table 1.

C. FLYASH

Fly ash is a waste material obtained as a result of coal combustion. Fly ash is bought from the local market. The percentage variation of fly ash is 5%, 10%, 15% and 20%. The composition of Fly ash is listed below in Table 2.

D. QUARRY DUST.

Quarry dust is waste material that is available in abundance. It is obtained from aggregate crusuing industries. The percentage variation for quarry dust is 5%, 15%, and 25%. The sample is collected from the local market. The composition of Quarry Dust is listed below in Table 3.

III.TABLE I PROPERTIES OF LIME

Calcium Oxide, CaO (%)	73.22
Loss on Ignition, LOI (%)	24.35

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Magnesium Oxide, MgO (%)	0.74
Ferric Oxide, Fe2O3 (%)	0.17
Aluminium Oxide, (%)	0.11
Calcium Sulphate, CaSO4 (%)	0.12
Phosphorus Oxide, P2O5 (%)	0.08

IV. TABLE III PROPERTIES OF FLY ASH

Specific gravity, G	2.05
Bulk Density g/cm 3	1.51
4 OMC (%)	24.5
AASHTO Classification	A-4
Absorption (%)	1.9
Max Dry Density g/cm3	12.4

V. TABLE IIIII PROPERTIES OF QUARRY DUST

Specific gravity, G	2.75
Bulk Density g/cm ³	1.8
Max Dry Density g/cm ³	1.97
AASHTO Classification	A-4
Absorption (%)	1.5
Specific gravity, G	2.75

VI.EXPERIMENTS

The following experiments were conducted to study the behavior of sample.

Liquid Limit(LL)-IS2720 PART V-1985, Plastic

Limit(PL)-IS2720 PART VI-1985, Specific Gravity

			-				
test- IS 2720 PART III-1985, Shrinkage limit (SL)							
, ₆ , ,	CLAY	CBR=4.4 Liquid limit-26.85					
Free Swell Index, Gradation (Sieve Analysis)	SAMP	SAMP OMC=23 Plastic limit-3.85					
Hydrometer analysis, Modified Proctor Compaction	LE						
jai in jai, in in in in in i	Lime	Fly	Qua	0	С	Liqui	Plastic
test- IS 2720 PART VIII-1985, Unconfined		ash	rry	Μ	В	d limit	-limit
Compaction Test and California Bearing Ratio	•		dust	С	R		
				%	%		
(CBR)test-IS 2720 part XVI-1987.	6	5	5	24	5.3	20.69	26.32
RESULTS AND DISCUSSIONS		5	15	21	6.7	15.45	14.63
		5	25	17	12.8	18.62	15.38
Sieve analysis		10	5	22	6.0	21.66	19.47
IRIAL I:		10	15	23	4.6	20.15	17.07
		15	5	21	9.1	8.834	14.66
		15	15	23	45	7 90	18 75

IS	Wt.	%	Cumulative %	%
sieve	Retained	retained	retained on	finer
size	in each	in each	each sieve	
	sieve	sieve		
(mm)	(g)			
4.75	0.012	2.4	2.4	97.6
2.36	0.064	12.8	15.2	84.8
1.18	0.086	17.2	32.4	67.6
0.6	0.1.6	21.2	53.6	46.4
0.3	0.106	21.2	74.8	25.2
0.15	0.062	12.4	87.2	12.8
0.075	0.05	10	97.2	2.8
Pan	0.014	2.8	100	0

TRIAL 2:

IS	Wt.	%	Cumulative %	%
sieve	Retained	retained	retained on	finer
size	in each	in each	each sieve	
	sieve	sieve		
(mm)	(g)			
4.75	0.01	2	2	98
2.36	0.082	16.4	18.4	81.6
1.18	0.102	20.4	38.8	61.2
0.6	0.114	22.8	61.6	38.4
0.3	0.096	19.2	80.8	19.2
0.15	0.058	11.6	92.4	7.6
0.075	0.028	5.6	98	2
Pan	0.01	2	100	0

 $\begin{array}{l} C_u = D_{60}/D_{10} = 1.064/0.148 = 7.19 \\ C_c = D_{30}^{2/}(D_{60} * D_{10}) {=} 0.416^{2}/(1.064 * 0.148) {=} 1.098 \\ \\ \textbf{SIEVE ANALYSIS FOR QUARRY DUST:} \\ Weight of the sample taken = 1 Kg \end{array}$

Specific gravity

peem	e granny				
TRIAL	$W_1(Kg)$	W_2 (Kg)	W ₃ (Kg)	W ₄ (Kg)	Gs
1	0.678	1.234	1.863	1.512	2.71
2	0.678	1.244	1.871	1.512	2.73
					-

VII. TABLE IVII CBR TEST RESULTS

24

4.6

20

5

18.28

22.76

CONCLUSION

The experimental study on behavior of black cotton soil is successfully completed and the following inferences were made:

- It is found that cohesive nature of BC soil decreases with increase in quantity of Quarry dust.
- The optimum moisture content for the sample mixture containing 6% lime, 5% fly ash and 25% quarry dust is found to be 17% which is comparatively lesser as when compared to normal sample being 23%.
- The CBR value for the same is found to be 3 times the sample obtained.
- This reaction is obtained as a result of pozzolanic interaction between lime, flyash and quarry dust with the clay minerals.
- As per IRC recommendations this sample mix can be employed for stabilizing rural roads, in subgrade.

In comparison IRC-37, the flexible pavement thickness can be reduced up to 45% of its original thickness. Thus the stabilization when adopted for flexible pavement would be cost efficient and effective.

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