Compressibility Parameters in Assessing the Collapse Potential of Red Soils in Visakhapatnam Region

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Abstract — Soils of Visakhapatnam region is located by red soil found in warm temperature, moist and deciduous weather and not capable of retaining moisture. These soils are red in colour due to high iron content and also show relatively high strength in dry state. Several methods have been studied on the degree of collapsibility by measuring collapsible potential. In the present study the volume changes are determined by conducting single oedometer test on compacted soils and their collapsible potential is classified as per Jennings and Knight (1975), ASTM D5333.

Keywords — Degree of collapsibility, single oedometer test, collapsible potential,

I. INTRODUCTION

Collapsible behavior of red soils can be considered as critical issue for geotechnical engineers in understanding challenges when structure founded on these soils subjected to large differential settlements which increase the cost of repairs and rehabilitation.

Exciting methods of Estimation of collapse potential:

1. Jennings and Knight (1975):

Jennings and knight describes the procedure for the single oedometer test to measure the collapse potential of soils.

Collapse Potential $C_P = (\Delta H/H)$

$$C_P = 0 - 1\%$$
 \rightarrow No Problem,

$$C_P = 1 - 5\%$$
 \rightarrow Moderate Trouble,

 $C_P = 5 - 10\%$ \rightarrow Trouble,

 $C_P = 10 - 20\%$ \rightarrow Severe trouble,

 $C_P = 0 - 1\%$ \rightarrow Very severe trouble. 2. ASTM D5333:

ASTM D5333 describes the procedure for the collapse potential (I_e), which is the wetting induced strain measured at a reference stress level of 200 kPa. Based on the value of the value of the collapse index, the degree of specimen collapse (or collapse potential) can be classified as:

Collapse potential $I_e = \Delta e/(1+e_o)$.

$I_e \leq 0$	\rightarrow Free from collapse
$I_e = 0.1$ to 2	\rightarrow Slight Collapse
$I_e = 2.1$ to 6	\rightarrow Moderate
$I_e = 6.1$ to 10	\rightarrow Moderate to Severe
$I_{e} > 10$	\rightarrow Severe
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Collapsible soils are quite extensive though identifications and characterization is not that simple. A more general approach may require assessing the collapsible potential.

II. MATERIALS

To study the geotechnical characterization and single oedometer of red soils in Visakhapatnam region, the soil samples were collected at a depth of 1.0 - 1.5 m from the ground level and the collected samples were dried and subjected for the strength tests as per IS code.

III. TESTS & RESULTS

To explain collapse potential of red soils, Ten number of SM nature with varying composition subjected to remoulded conditions to their corresponding bulk density, dry densities, water content, void ratio, there collapse potential from Jennings and knight and ASTM D5333.

Location/Property	SM-I	SM- II	SM- III	SM- IV	SM- V	SM- VI	SM- VII	SM- VIII	SM- IX	SM-X
Gradation Properties										
Gravel (%)	0	0	0	0	0	0	0	0	0	0
Sand (%)	85	80	76	82	74	78	76	75	80	82
Fines (%)	15	20	24	18	26	22	24	25	20	18

Silt (%)	15	17	20	16	21	19	20	19	14	14
Clay (%)	0	3	4	2	5	3	4	5	6	4
Specific Gravity	2.65	2.65	2.65	2.65	2.65	2.65	2.65	2.65	2.66	2.65
			Inc	lex Prop	erties					
Liquid Limit (%)	21	22	23	22	21	22	22	21.50	23.5	22
Plastic Limit (%)	18	18	19	18	17	19	18	17.50	19.5	18.50
Plasticity Index (I _p)	3	4	4	4	4	3	4	4	4	3.50
IS Classification	SM	SM	SM	SM	SM	SM	SM	SM	SM	SM
			Compac	tion Cha	racterist	ics				
OMC (%)	9.00	9.20	9.40	9.00	8.80	9.30	9.20	9.00	9.50	9.10
MDD (g/cc)	1.75	1.74	1.77	1.75	1.68	1.72	1.73	1.70	1.76	1.71
Bulk Density (g/cc)	1.54	1.56	1.55	1.54	1.50	1.53	1.54 to	1.48 to	1.53	1.52 to
	to	to	to	to	to	to	1.89	1.85	to	1.86
	1.90	1.90	1.94	1.91	1.83	1.88			1.93	
Dry Density (g/cc)	1.49	1.51	1.50	1.50	1.45	1.48	1.48 to	1.44 to	1.47	1.48 to
	to	to	to	to	to	to	1.73	1.70	to	1.71
	1.75	1.74	1.77	1.75	1.68	1.72			1.76	
Water Content (W _n)	3.20	3.60	3.20	3.00	3.20	3.40	3.80 to	3.00 to	4.00	2.90 to
	to	to	to	to	to	to	9.20	9.00	to	9.10
	9.00	9.20	9.40	9.00	8.80	9.30			9.50	
Void ratio	0.78	0.76	0.77	0.77	0.83	0.79	0.79 to	0.84 to	0.81	0.79 to
	to	to	to	to	to	to	0.53	0.55	to	0.54
	0.52	0.52	0.50	0.51	0.58	0.54			0.51	

- ➢ Grain size distribution of red soils are dominated by sand particles of 74 − 85%, fines of 15 − 26%, out of which silt particles are 14 − 21% and clay particles are 0 − 6%.
- These soils have liquid limit in the range of 21% - 23.5%, plasticity index in the range of 3-4. As per I.S 1498-1970 these are classified under SM Soil.
- Red soils can attain high density and bearing values at optimum moisture content can be effectively used in civil engineering constructions.

A. PARAMETERS CONSIDERED IN EXPLAINING COLLAPSIBLE POTENTIAL:

To explain the collapse potential (C_P) of red soil in Vishakhapatnam region of SM nature by considering the parameters mentioned above at their remoulded conditions are shown in below. Ten number of red soils of SM group were considered and their corresponding, compression (settlement), percentage volume decrease and collapse potential are grouped and are shown below.

$\Upsilon_{d}(g/cc) \rightarrow$	1.4 – 1.5	1.5 – 1.6	1.6 – 1.7	1.7 – 1.8					
Soils	Compression ∆H								
SM - I	0.20	0.18	0.09	0.02					
SM - II	0.20	0.13	0.04	0.01					
SM - III	0.22	0.18	0.07	0.01					
SM - IV	0.23	0.19	0.04	0.00					
SM - V	0.25	0.13	0.03	0.00					
SM - VI	0.19	0.10	0.04	0.01					
SM - VII	0.19	0.08	0.03	0.01					
SM - VIII	0.24	0.13	0.04	0.01					
SM - IX	0.23	0.15	0.04	0.01					
SM - X	0.21	0.13	0.04	0.01					
Range	0.19 - 0.24	0.08 - 0.19	0.03 - 0.09	0.00 - 0.02					

Table: 2 Variation of Compression with dry density

$\Upsilon_{d}(g/cc) \rightarrow$	1.4 – 1.5	1.5 – 1.6	1.6 – 1.7	1.7 – 1.8					
Soils 1	Percentage Volume Decrease								
SM - I	10.00	9.00	4.50	1.00					
SM - II	10.00	6.50	2.00	0.50					
SM - III	11.00	9.00	3.50	0.00					
SM - IV	11.50	9.50	2.00	0.00					
SM - V	12.50	6.50	1.00	0.50					
SM - VI	9.50	5.00	2.00	0.50					
SM - VII	9.50	4.00	1.50	0.50					
SM - VIII	12.00	6.50	2.00	0.50					
SM - IX	11.50	7.50	2.00	0.50					
SM - X	10.50	6.50	2.00	0.50					
Range	9.50-12.50	4.00-9.50	1.00-4.50	0.00-1.00					

Table: 2 Variation of Percentage volume decrease with dry density

 Table: 2 Variation of Collapse Potential with dry density

$\Upsilon_{d}(g/cc) \rightarrow$	1.4 – 1.5	1.5 – 1.6	1.6 – 1.7	1.7 – 1.8					
Soils	Collapse Potential								
SM - I	9.00	7.40	3.50	0.66					
SM - II	8.90	6.02	1.28	0.66					
SM - III	12.50	8.23	3.10	0.67					
SM - IV	10.73	8.72	1.28	0.66					
SM - V	10.90	5.32	2.08	0.63					
SM - VI	8.40	4.14	1.25	0.65					
SM - VII	8.40	3.60	1.25	0.65					
SM - VIII	12.00	5.30	2.00	0.50					
SM - IX	10.50	7.10	1.30	0.66					
SM - X	9.50	5.30	1.30	0.65					
Range	8.40-12.50	3.60-8.72	1.28-3.50	0.50-0.67					

B. COMPARISON OF SINGLE OEDOMETER TEST RESULTS:

Based on the test results of single oedometer test conducted on red soils compacted at varies dry densities as per ASTM D5333 the following identifications are made.

a) Soils compacted at densities in between 1.4g/cc – 1.5g/cc:

Red soils of SM nature compacted at dry densities of (1.4-1.5g/cc) exhibited the compression (settlements)in the range of 0.19-0.24 and percentage volume decrease in the range of 9.50-12.50. At these dry densities the Jennings and Knight (Cp) is in the range of 8.40-12.50, which is in between 5-20% classified under severe trouble to trouble, similarly as per ASTM D5333 these are greater than 10%, and classified under moderately severe too severe.

b) Soils compacted at densities in between 1.5g/cc – 1.6g/cc:

Red soils of SM nature compacted at dry densities (1.5-1.6 g/cc) exhibited the compression (settlement)in the range of 0.08-0.19and percentage volume decrease in the range of 4.00-9.50. At these dry densities the Jennings and Knight (Cp) is in the range of 3.60-8.72, which is in between 1-10% classified under moderate trouble to trouble, similarly as per ASTM D5333 which are in the range of 2.1-10%, and classified under moderate collapse to moderately severe.

c) Soils compacted at densities in between 1.6g/cc – 1.7g/cc:

Red soils of SM nature compacted at dry densities (1.6-1.7 g/cc) exhibited the compression (settlement) in the range of 0.03-0.09 and percentage volume decrease in the range of 1.00-4.50. At these dry densities the Jennings and Knight (Cp) is in the range of 1.28-3.50, which is in between 1-10% classified under moderate trouble to trouble, similarly ASTM D5333 which are in the range of 2.1-10%, and

classified under moderate collapse to moderately severe.

d) Soils compacted at densities in between 1.7g/cc – 1.8g/cc:

Red soils of SM nature compacted at dry densities (1.7-1.8 g/cc) exhibited the compression (settlement) in the range of 0.00-0.02and percentage volume decrease in the range of 0.00-1.00. At these dry densities the Jennings and Knight (Cp) is in the range of 0.50-0.67, which is in between 0-1% classified under no problem, similarly ASTM D5333 which are in the range of 0.1 -2 %, and classified under slight collapse.

IV. CONCLUSION

- 1. Red soils compacted at dry densities in the range of 1.4-1.5 g/cc exhibited high collapse potential under saturated condition.
- 2. Red soils compacted at dry densities nearing to MDD exhibited low collapse potential, dry densities between dry to maximum dry density conditions exhibited moderate state of collapse behavior.
- 3. Jennings and Knight (1975), ASTM D5333, have the same trends in assessing the collapse potential of red soils.

V. REFERENCES

[1] ASTM Standard D -5333, 2003: "Standard Test Methods for Measurement of Collapse Potential of Soils", Annual Book of ASTM international, West Conshohocken, PA.

- [2] IS 1498–1970.classification of soils.
- [3] IS 2720: Part 15: 1965 Methods of test for soils –Part 15: Determination of consolidation properties.
- [4] IS 2720: Part 3: Sec 2:1980 Methods of Test for Soils –Part III: Determination of Specific Gravity –Section 2: Fine, Medium and coarse Grained Soils.
- [5] IS 2720: Part 4: 1985 Methods of test for soils –Part 4: Grain sieve analysis.
- [6] IS 2720: Part 5: 1985 Methods of test for soils –Part 5: Determination of liquid limit and plastic limit.
- [7] Jennings JE, Knight K. (1957) the additional settlement of foundations due to collapse of sandy soils on wetting. In: Proceedings of the 4th international conference on soil mechanics and foundation engineering, London; 1957. p. 316-9.
- [8] Jennings, J.E and Knight, K. (1975): "A guide to construction on or with materials exhibiting additional settlement due to collapse of grain structure." Proceedings, 6thregional Conference for Africa on Soil Mechanics and Foundation Engineering, Durban, South Africa, Vol.1, pp.99-105.
- [9] K. Saroja Rani, P.V.V.Satyanarayana, "Collapse Behaviour of Red Soils of Silty Sand Nature Based on Engineering Properties", International Journal of Civil, Structural, Environmental and Infrastructure Engineering Research and Development (IJCSEIERD) ISSN (P): 2249-6866; ISSN (E): 2249-7978 Vol. 9, Issue 1, Feb 2019, 11-14 @ TJPRC Pvt. Ltd.
- [10] K. Saroja Rani, P.V.V. Satyanarayana, G.Mounika, "Affect of a In-Situ Parameters in Explaining Collapsible Behaviour of Red Soils." IOSR Journal of Engineering (IOSR-JEN) e-ISSN: 22781684,p-ISSN: 2320-334X, Volume 9, Issue 1Ver. II (Jan 2019), PP 09-13.
- [11] K. Saroja Rani, P.V.V. Satyanarayana, "Role of Engineering Properties in Predicting Collapsible Behaviour of Red Soils" SSRG International Journal of Civil Engineering (SSRG – IJCE) - Volume 5, Issue 12, December 2018.